

September 18, 2002

Ahmed Elgamal

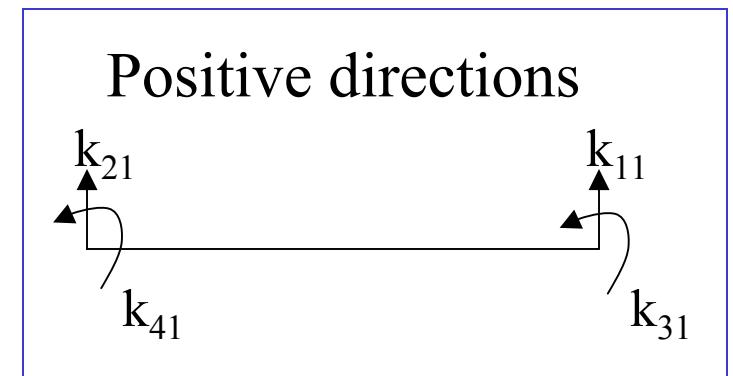
# **Stiffness Coefficients for a Flexural Element**

**Ahmed Elgamal**

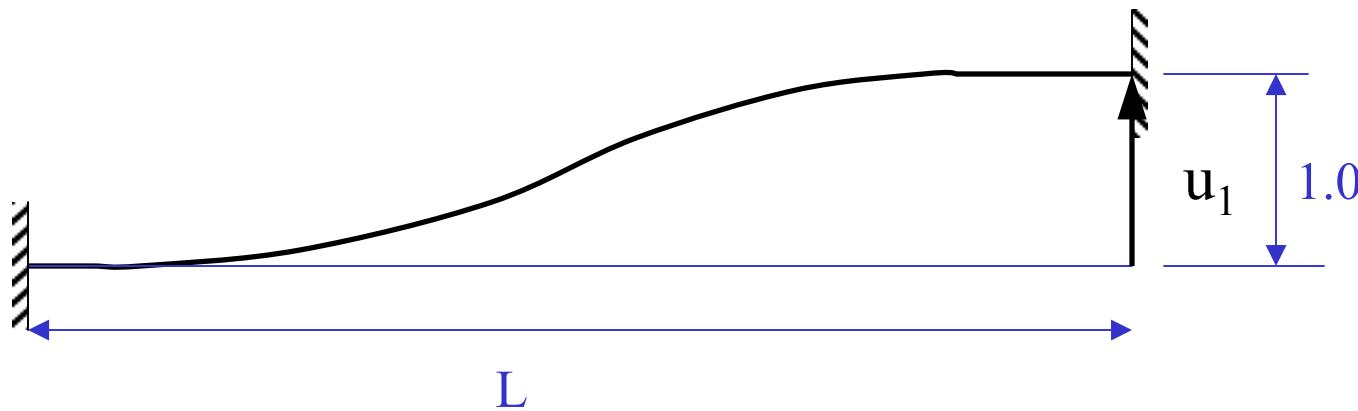
Stiffness coefficients for a flexural element (neglecting axial deformations), Appendix 1, Ch. 1 Dynamics of Structures by Chopra.



4 degrees of freedom



To obtain k coefficients in 1<sup>st</sup> column of stiffness matrix, move  $u_1 = 1$ ,  $u_2 = u_3 = u_4 = 0$ , and find forces and moments needed to maintain this shape.



These are (see structures textbook)

$$\begin{array}{c} \text{---} \\ | \\ \text{---} \\ \leftarrow \quad \downarrow \\ \frac{6EI}{L^2} \\ \frac{12EI}{L^3} \end{array}$$

Note that  $\Sigma$  Forces = 0

$\Sigma$  Moments = 0

$$\Sigma M = \frac{12EI}{L^3} - \frac{12EI}{L^3} = 0$$

i.e. remember  $\frac{12EI}{L^3}$ , and you  
can find other forces & moments

Positive directions

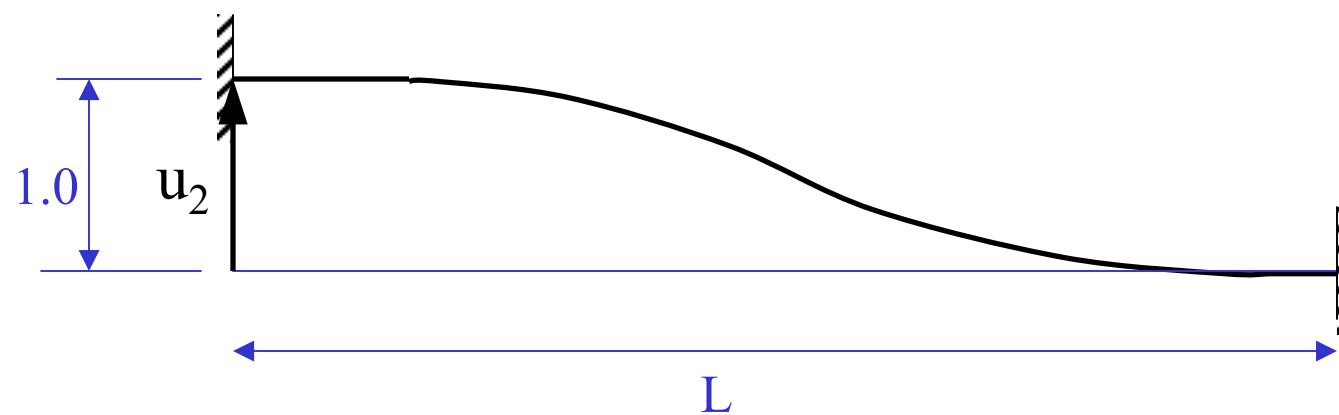


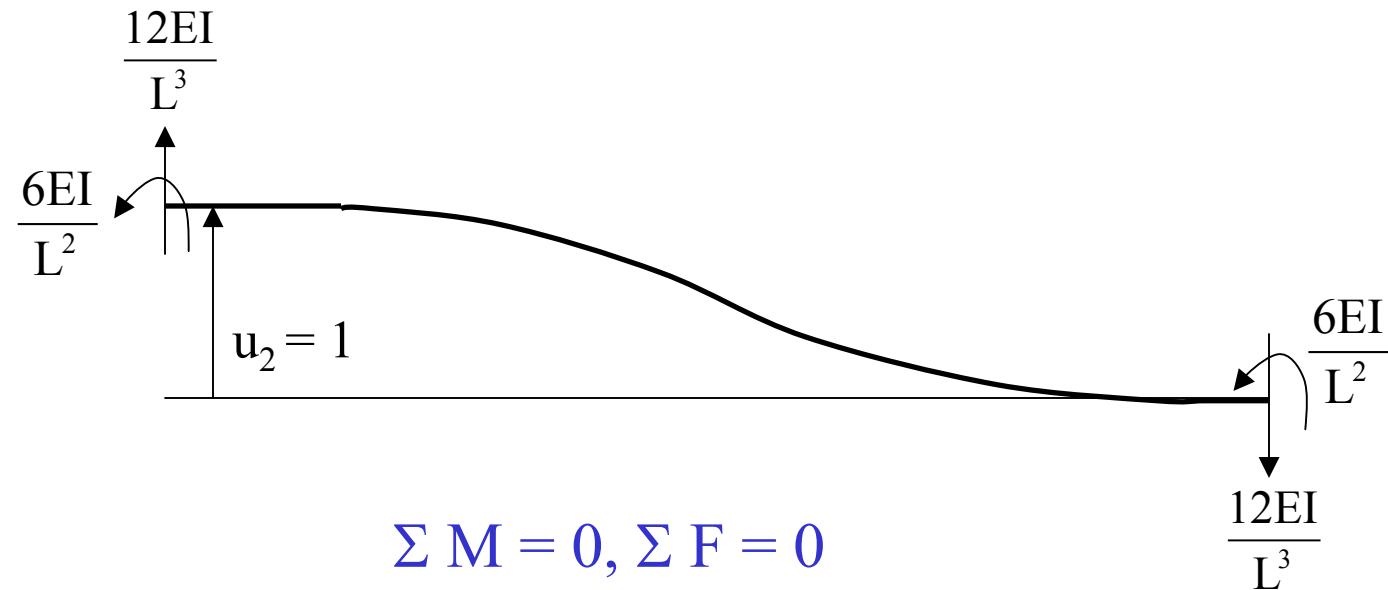
$$\underline{k} = \begin{bmatrix} k_{11} & k_{12} & k_{13} & k_{14} \\ k_{21} & k_{22} & k_{23} & k_{24} \\ k_{31} & k_{32} & k_{33} & k_{34} \\ k_{41} & k_{42} & k_{43} & k_{44} \end{bmatrix}$$

$k_{ij} = \underline{k}$ , where i is row number  
and j is column number

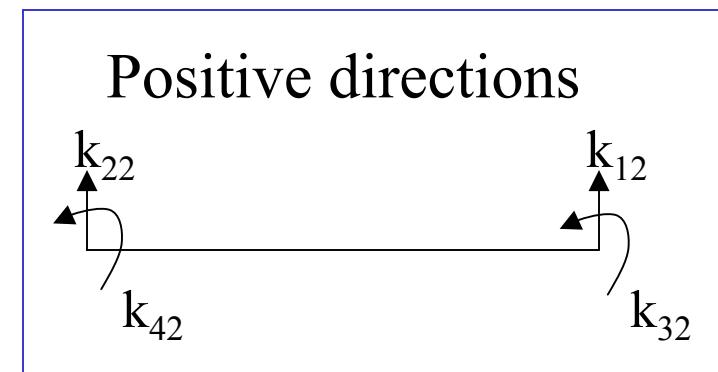
$$\underline{k} = \frac{EI}{L^3} \begin{bmatrix} 12 \\ -12 \\ -6L \\ -6L \end{bmatrix}$$

$$u_2 = 1, u_1 = u_3 = u_4 = 0$$

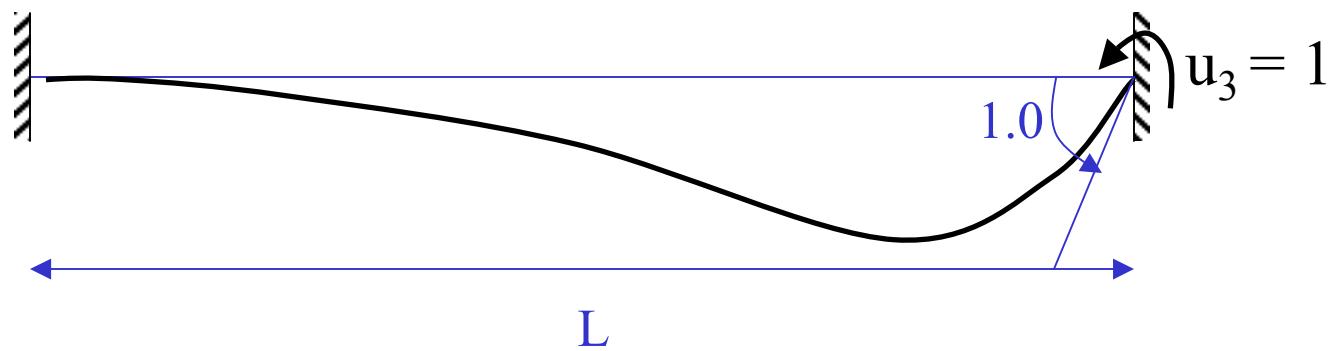


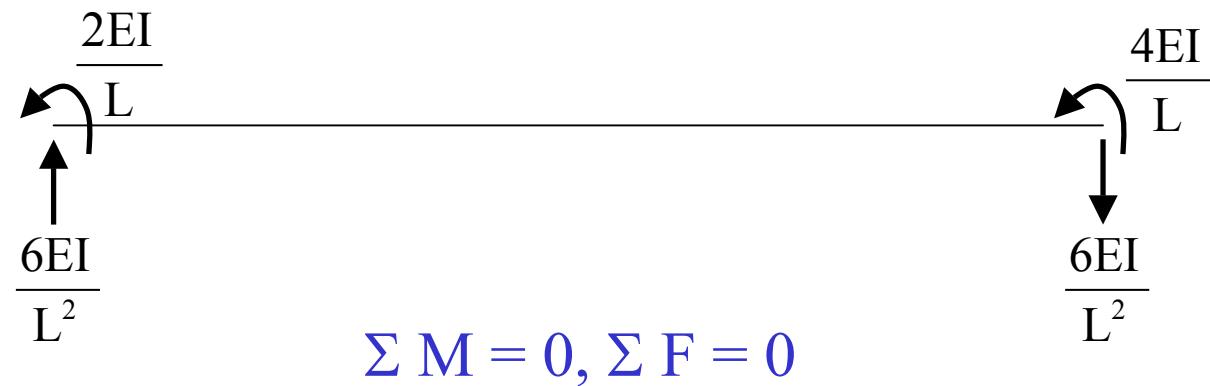


$$\underline{k} = \frac{EI}{L^3} \begin{bmatrix} -12 & 12 \\ 12 & 6L \\ 6L & 6L \end{bmatrix}$$

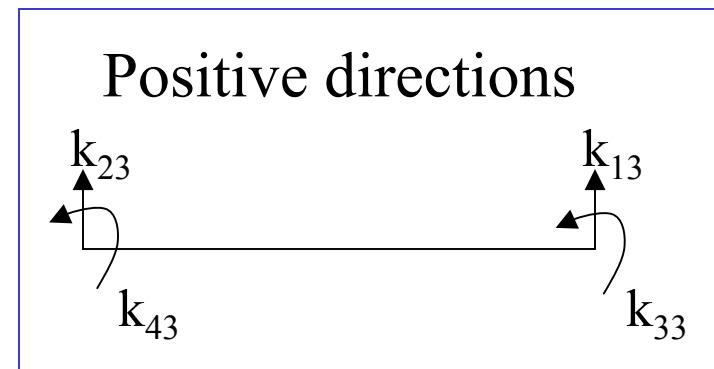


$$u_3 = 1, u_1 = u_2 = u_4 = 0$$

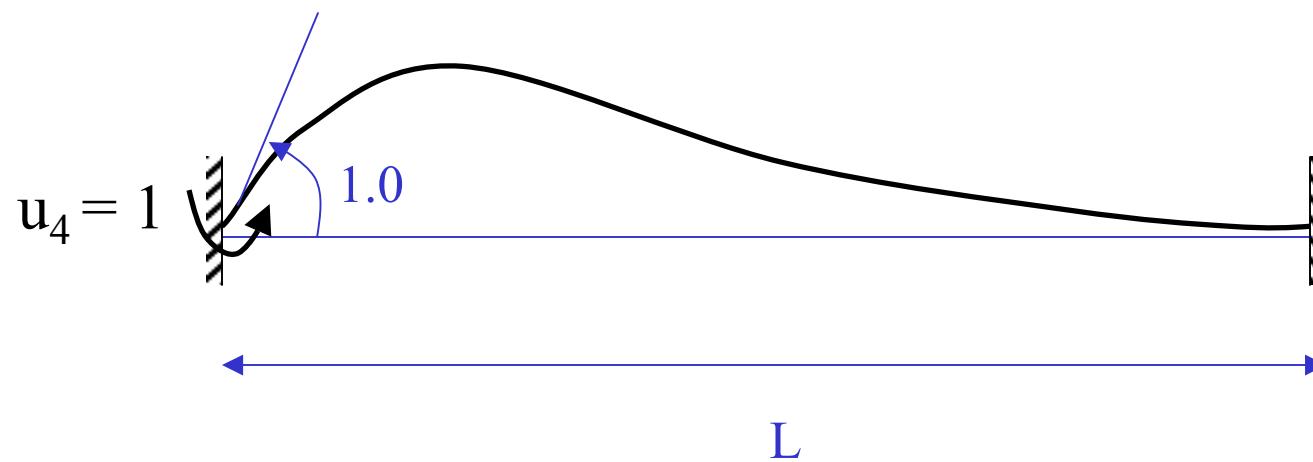


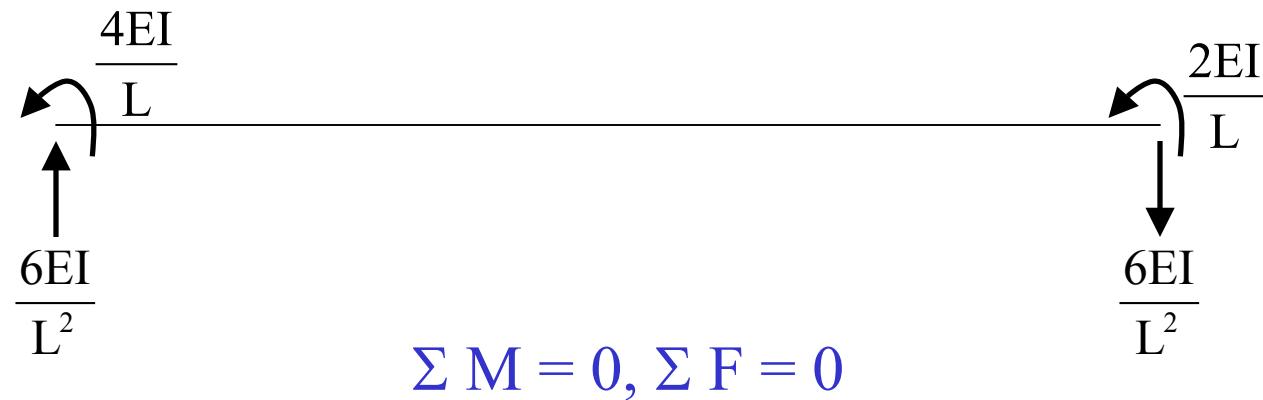


$$\underline{k} = \begin{bmatrix} -\frac{6EI}{L^2} \\ \frac{6EI}{L^2} \\ \frac{4EI}{L} \\ \frac{2EI}{L} \end{bmatrix}$$

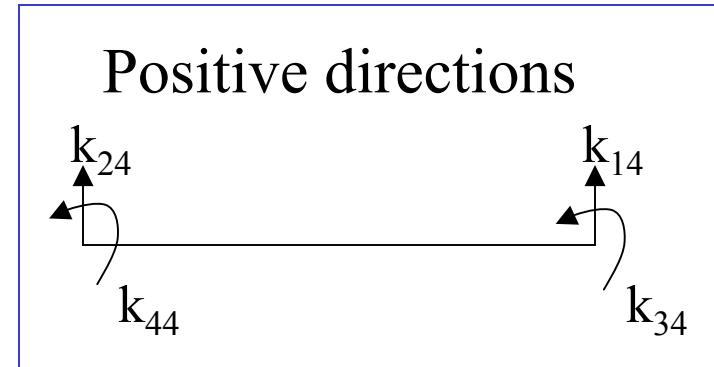


$$u_4 = 1, u_1 = u_2 = u_3 = 0$$

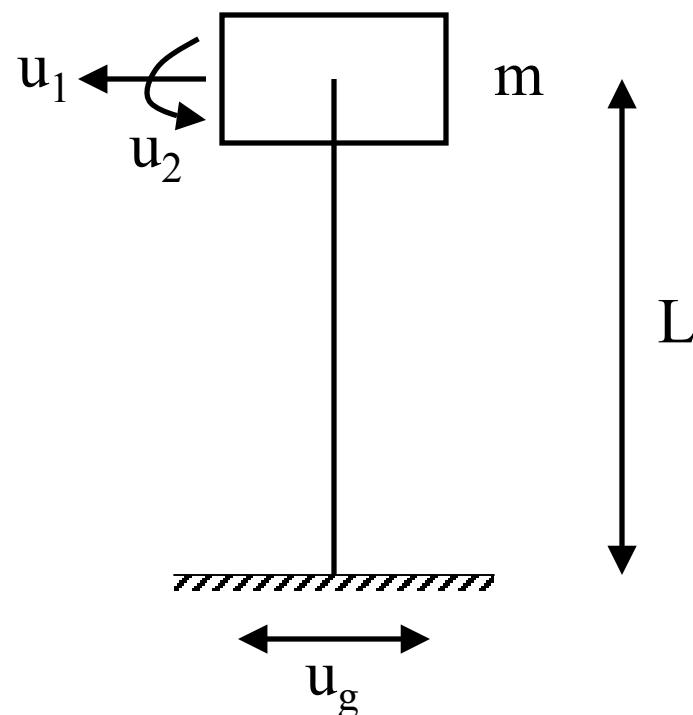




$$\underline{k} = \begin{bmatrix} -\frac{6EI}{L^2} \\ \frac{6EI}{L^2} \\ \frac{2EI}{L} \\ \frac{4EI}{L} \end{bmatrix}$$



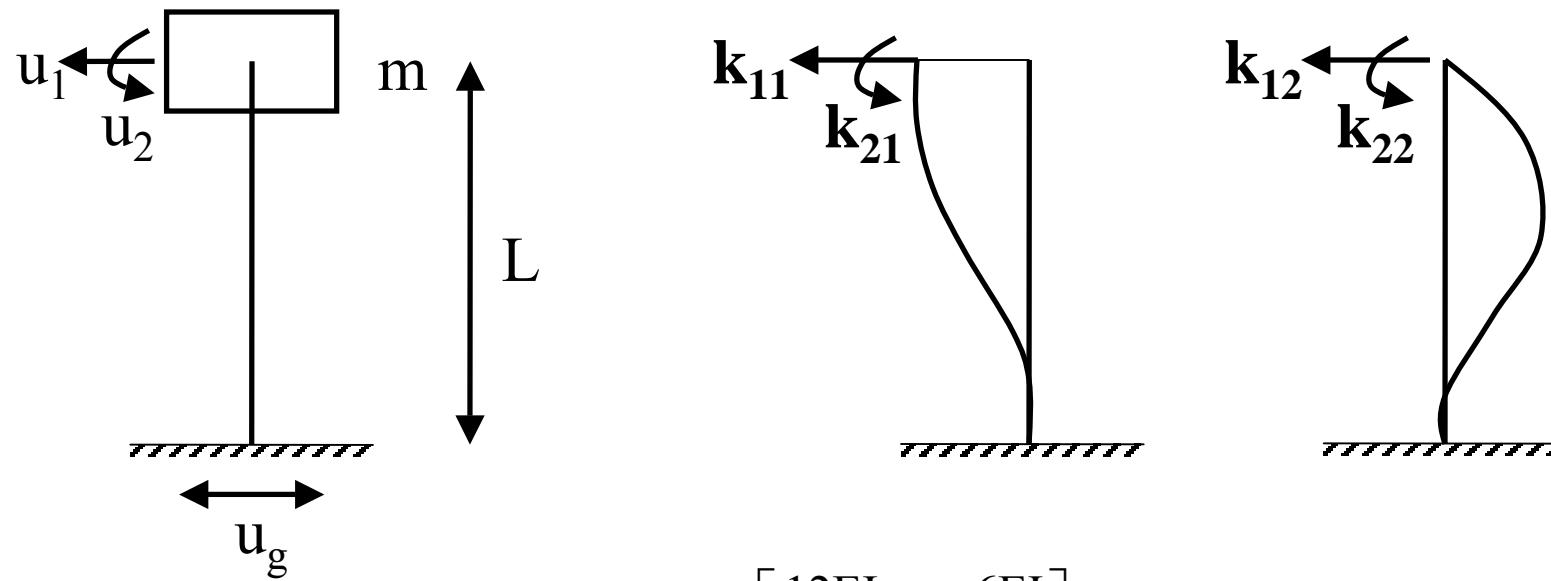
## Example: Water Tank



$m$  is lumped at a point & does not contribute in rotation

$u_2$  above was  $u_3$  in the earlier section of these notes

## Example: Water Tank (continued)



$$\begin{bmatrix} m & 0 \end{bmatrix} \begin{bmatrix} \ddot{u}_1 \\ \ddot{u}_2 \end{bmatrix} + \begin{bmatrix} \frac{12EI}{L^3} & \frac{-6EI}{L^2} \\ \frac{-6EI}{L^2} & \frac{4EI}{L} \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \end{bmatrix} = -\begin{bmatrix} m \\ 0 \end{bmatrix} \ddot{u}_g$$

“Note Symmetry”      Rotational  
(used to be  $u_3$ )

## Example: Water Tank (continued)

### Static Condensation:

Way to solve a smaller system of equations by eliminating degrees of freedom with zero mass.

e.g., in the above, the 2<sup>nd</sup> equation gives

$$\frac{-6EI}{L^2}u_1 + \frac{4EI}{L}u_2 = 0$$

or

$$u_2 = \frac{6EI}{L^2} \frac{L}{4EI} u_1 = \frac{6}{4L} u_1 = \frac{3}{2L} u_1 \quad ----- *$$

## Example: Water Tank (continued)

Substitute \* into Equation 1

$$m\ddot{u}_1 + \left( \frac{12EI}{L^3} - \frac{6EI}{L^2} \frac{3}{2L} \right) u_1 = -m\ddot{u}_g$$

or,

$$m\ddot{u}_1 + \left( \frac{24EI - 18EI}{2L^3} \right) u_1 = -m\ddot{u}_g$$

or,

$$m\ddot{u}_1 + \left( \frac{3EI}{L^3} \right) u_1 = -m\ddot{u}_g$$

Now, solve for  $u_1$  and  $u_2$  can be evaluated from Equation \* above.

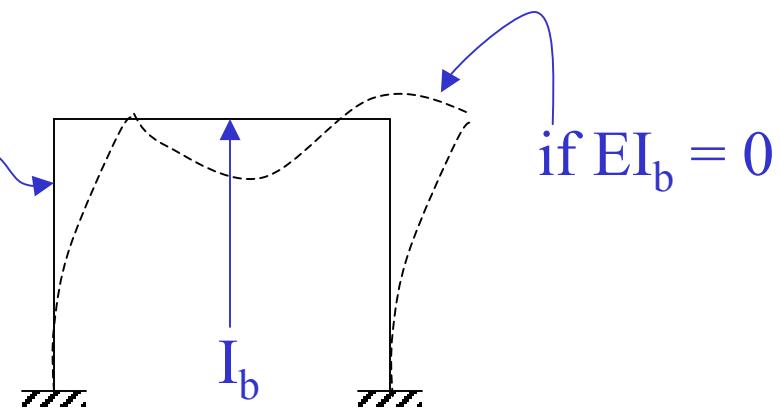
Static condensation can be applied to large MDOF systems of equations, the same way as shown above.

## Example: Water Tank (continued)

$$m\ddot{u}_1 + \left( \frac{3EI}{L^3} \right) u_1 = -m\ddot{u}_g$$

↑  
k of water tank as we were given earlier.

or of column

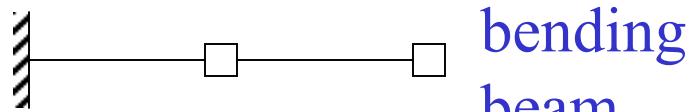


## Mandatory Reading

Example 9.4 page 362-364

Example 9.8 page 368-369

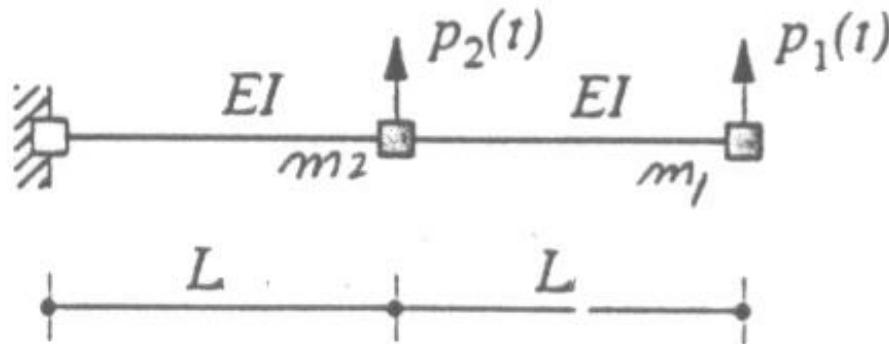
Sample Exercises: 9.5, 9.8, & 9.9



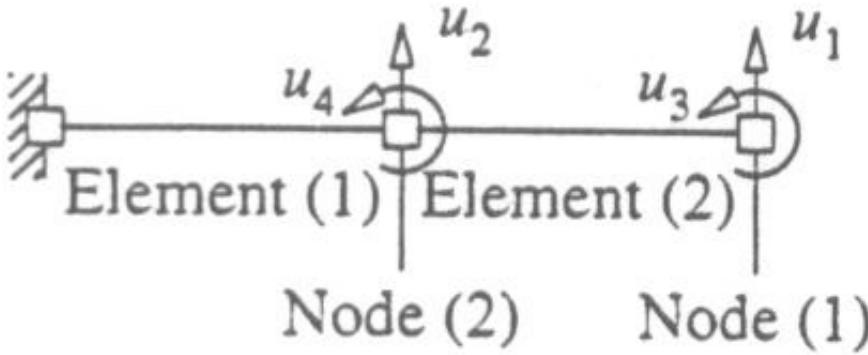
bending  
beam

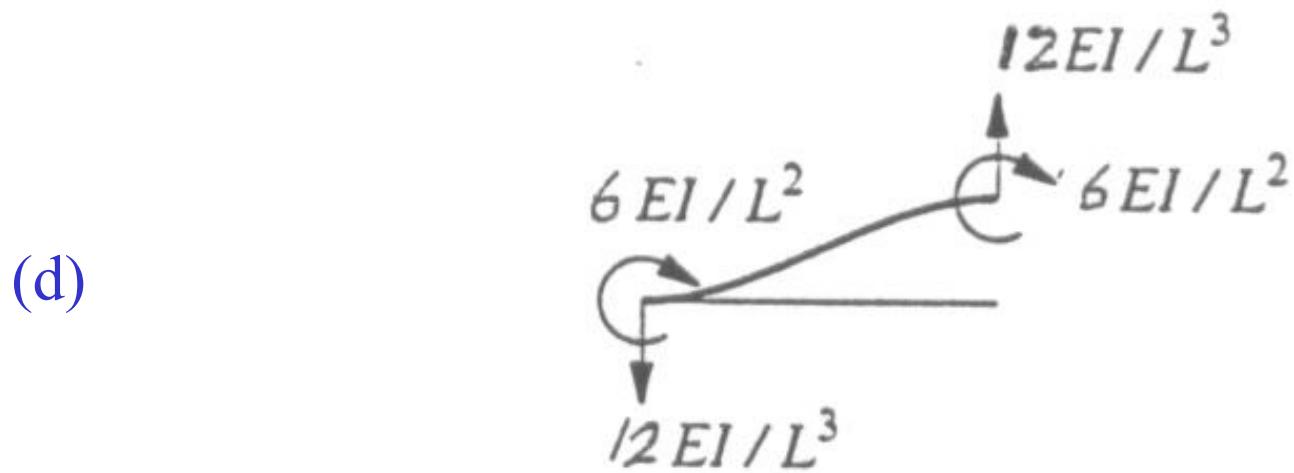
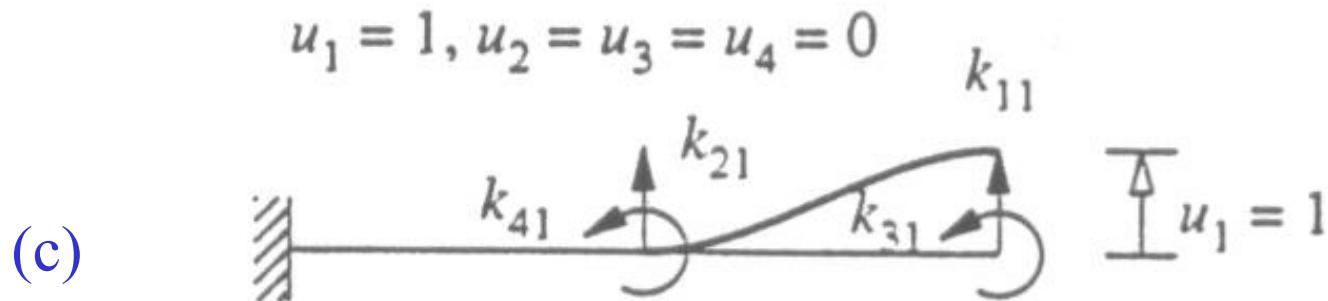
## Example

(a)

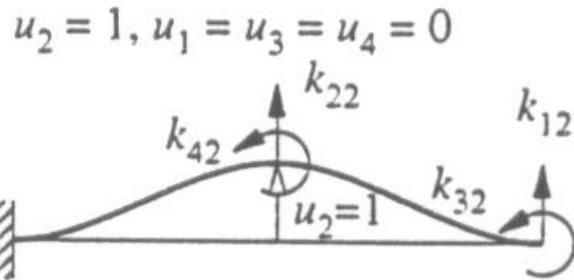


(b)

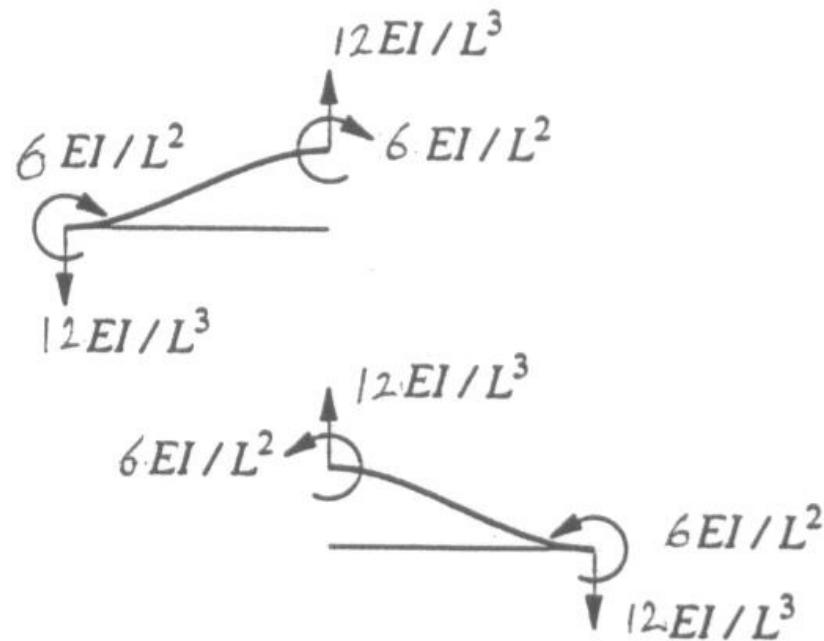




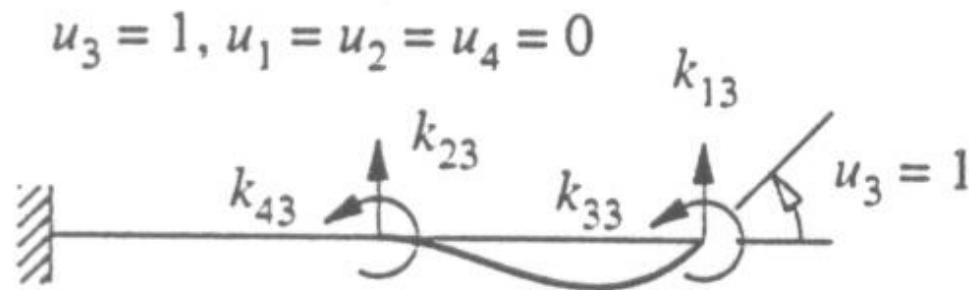
(e)



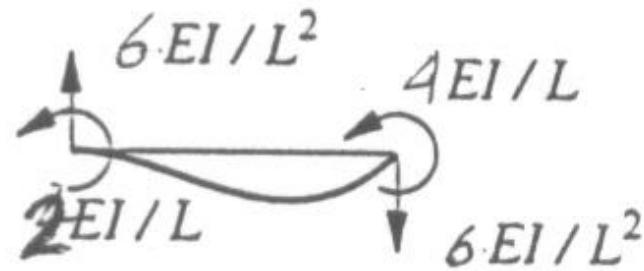
(f)



(g)

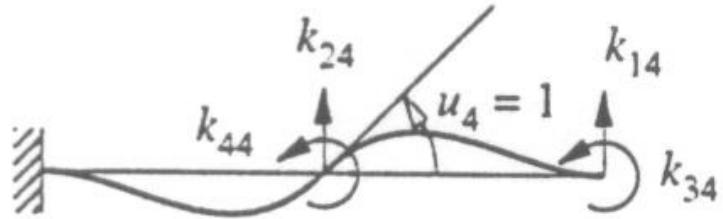


(h)

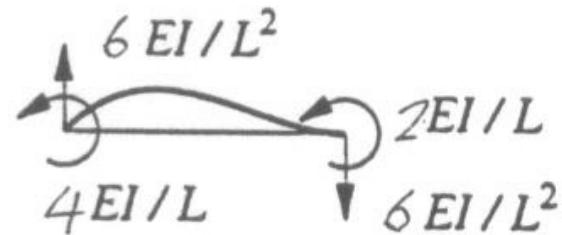
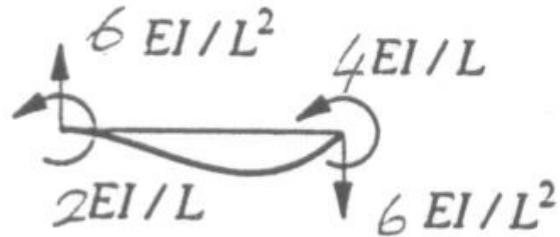


$$u_4 = 1, u_1 = u_2 = u_3 = 0$$

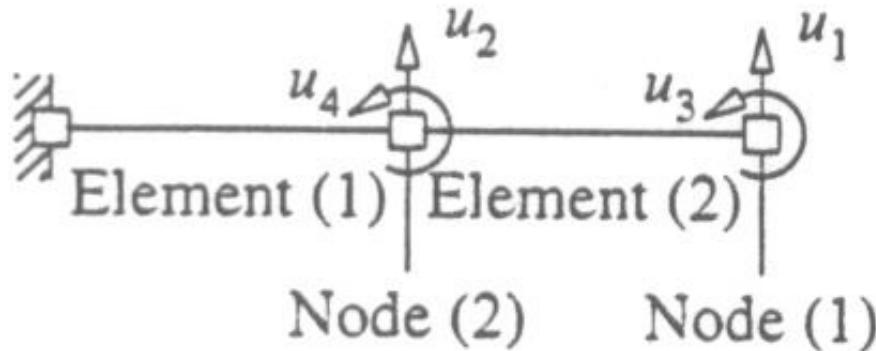
(i)



(j)



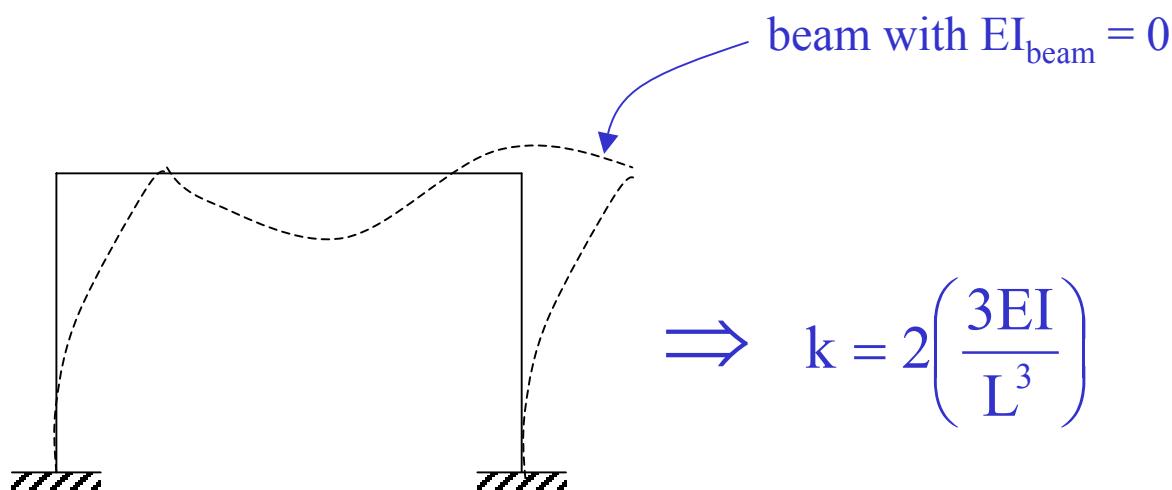
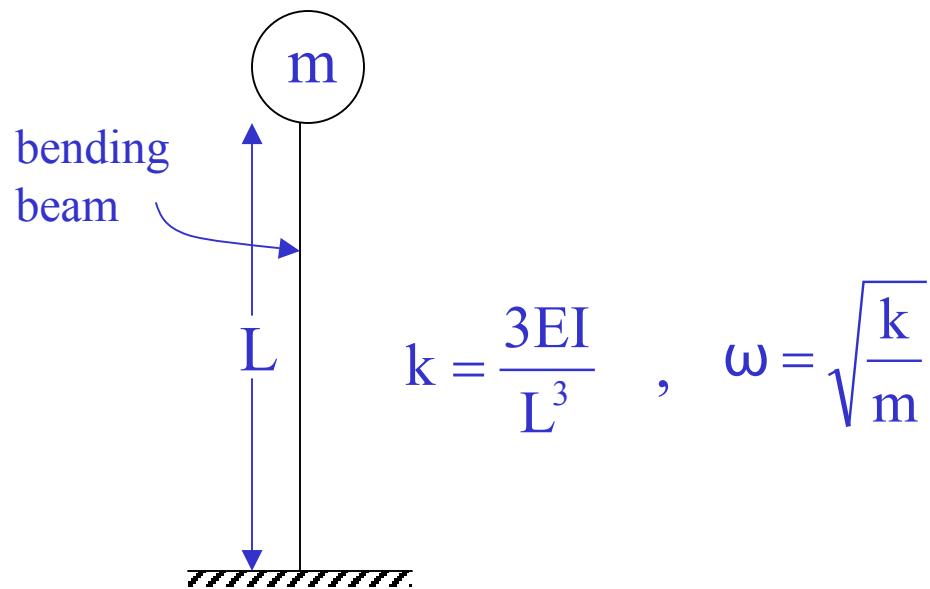
therefore,

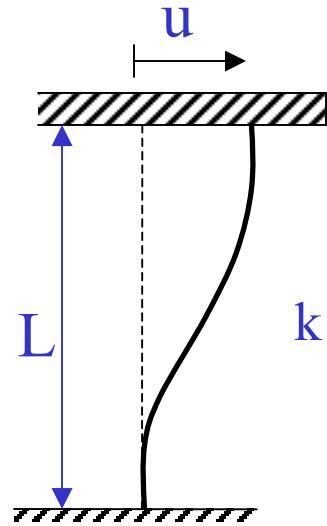


$$\underline{k} = \frac{EI}{L^3} \begin{bmatrix} 12 & -12 & -6L & -6L \\ -12 & 24 & 6L & 0 \\ -6L & 6L & 4L^2 & 2L^2 \\ -6L & 0 & 2L^2 & 8L^2 \end{bmatrix}, \quad \underline{M} = \begin{bmatrix} m_1 & & & \\ & m_2 & & \\ & & 0 & \\ & & & 0 \end{bmatrix}$$

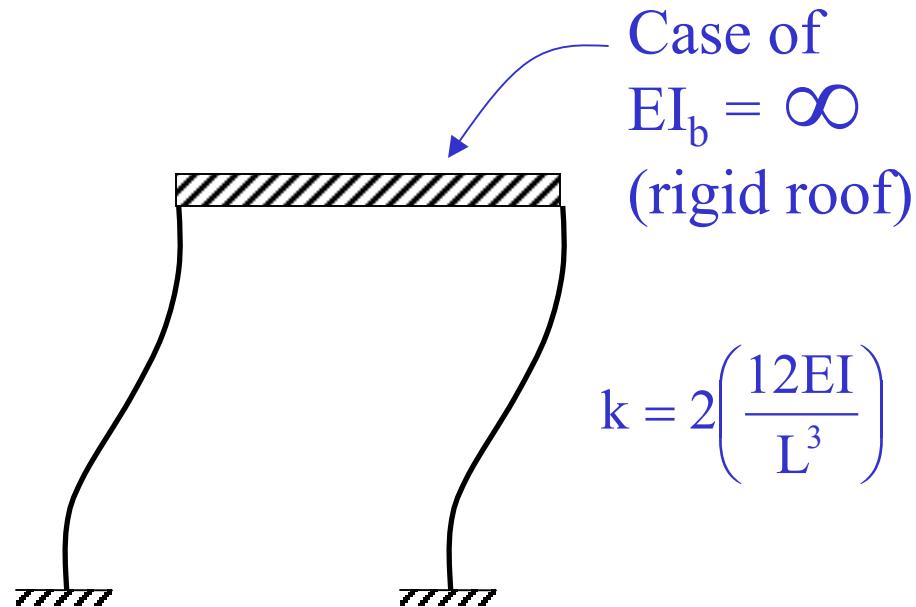
**Sample Exercise:** For the above cantilever system, write equation of motion and perform static condensation to obtain a 2 DOF system.

## Column Stiffness (lateral vibration)

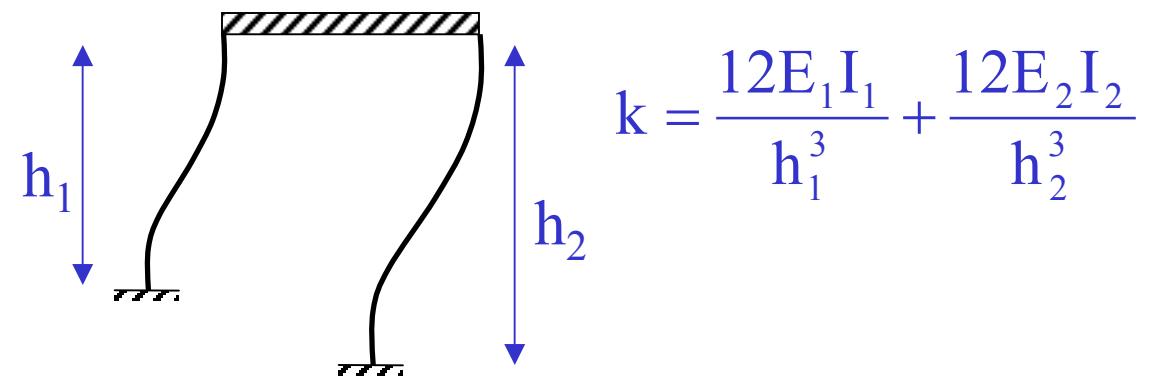




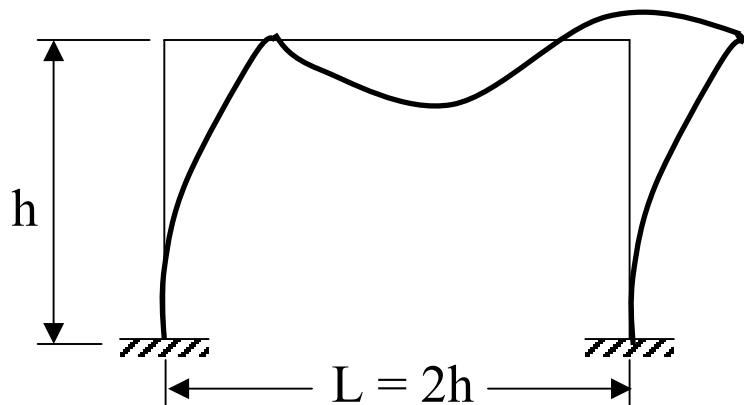
$$k = \frac{12EI}{L^3}$$



$$k = 2 \left( \frac{12EI}{L^3} \right)$$



$$k = \frac{12E_1 I_1}{h_1^3} + \frac{12E_2 I_2}{h_2^3}$$



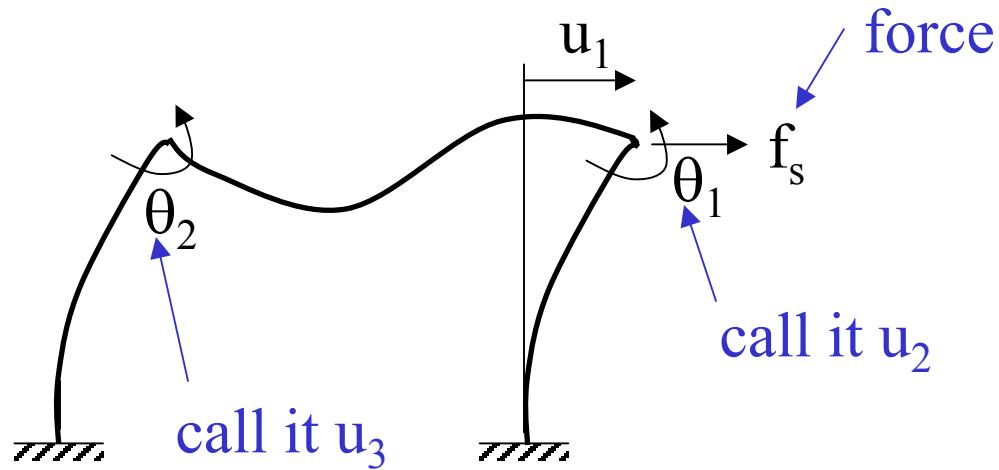
(See example 1.1 in  
Dynamics of Structures  
by Chopra)

$$k = \frac{96EI_c}{7h^3} \quad \text{if} \quad EI_b = EI_c$$

beam      column

$$k = \frac{24EI_c}{h^3} \frac{12\rho + 1}{12\rho + 4} , \quad \rho = \frac{I_b}{4I_c} \quad \& \quad E_c = E_b = E$$

Obtained by “static condensation” of 3x3 system



$$\begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = \begin{bmatrix} f_s \\ 0 \\ 0 \end{bmatrix}$$

Use to represent  $u_2$  and  $u_3$  in terms of  $u_1$  & plug back into

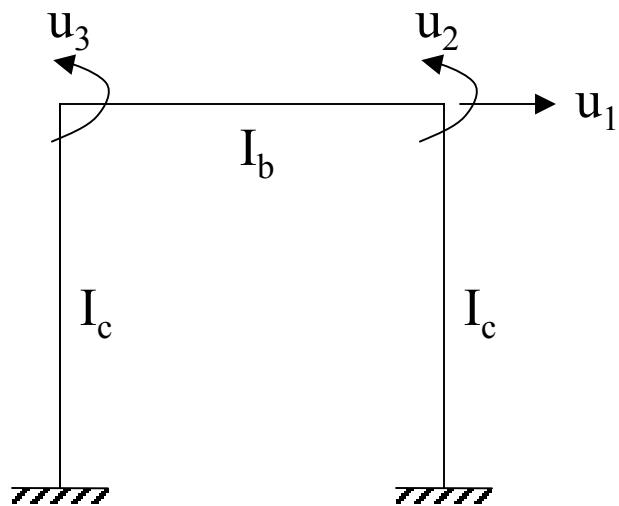
and get  $f_s = ku_1$

Technique can also be used for large systems of equations

(See example 1.1 in Dynamics of Structures by Chopra)

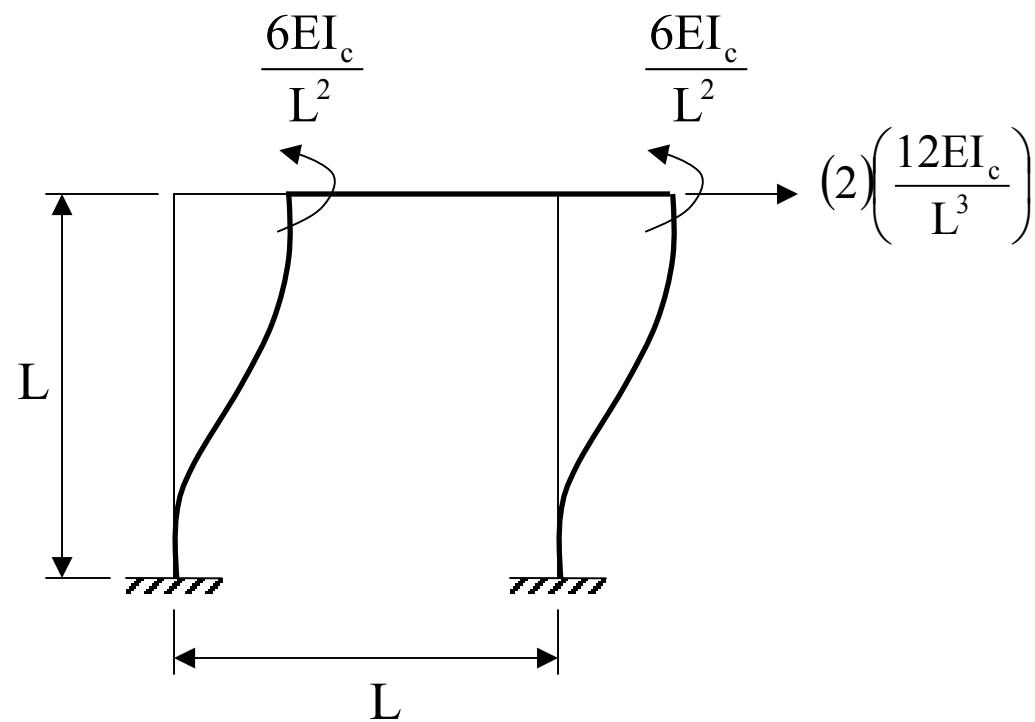
## Draft Example

Neglect axial deformation



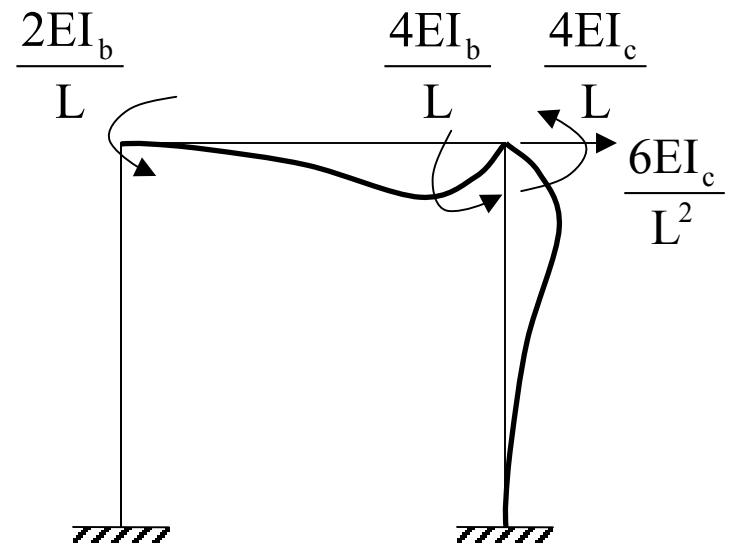
$$u_1 = 1$$

$$u_2 = u_3 = 0$$



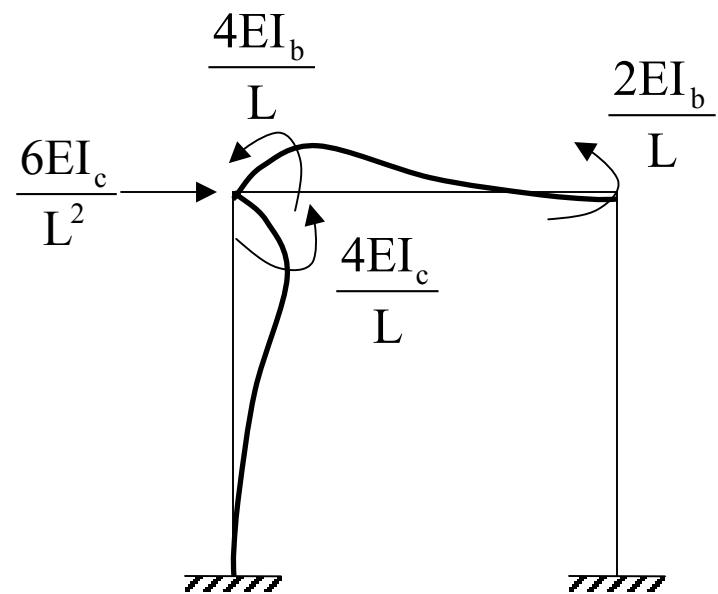
$$u_2 = 1$$

$$u_1 = u_3 = 0$$



$$u_3 = 1$$

$$u_1 = u_2 = 0$$



$$\underline{k} = \frac{E}{L^3} \begin{bmatrix} 24I_c & 6I_cL & 6I_cL \\ 6I_cL & 4(I_b + I_c)L^2 & 2I_bL^2 \\ 6I_cL & 2I_bL^2 & 4(I_b + I_c)L^2 \end{bmatrix}$$

If frame is subjected to lateral force  $f_s$

Then (for simplicity, let  $I_c = I_b = I$ )

$$\frac{EI}{L^3} \begin{bmatrix} 24 & 6L & 6L \\ 6L & 8L^2 & 2L^2 \\ 6L & 2L^2 & 8L^2 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = \begin{bmatrix} f_s \\ 0 \\ 0 \end{bmatrix}$$

Static condensation:

From 2<sup>nd</sup> and 3<sup>rd</sup> equations,

$$\begin{aligned} \begin{bmatrix} u_2 \\ u_3 \end{bmatrix} &= -\begin{bmatrix} 8L^2 & 2L^2 \\ 2L^2 & 8L^2 \end{bmatrix}^{-1} \begin{bmatrix} 6L \\ 6L \end{bmatrix} u_1 \\ &= \frac{-1}{64L^4 - 4L^4} \begin{bmatrix} 8L^2 & -2L^2 \\ -2L^2 & 8L^2 \end{bmatrix}^{-1} \begin{bmatrix} 6L \\ 6L \end{bmatrix} u_1 \\ &= \frac{-6}{10L} \begin{bmatrix} 1 \\ 1 \end{bmatrix} u_1 \end{aligned}$$

Note matrix inverse:

$$\begin{bmatrix} a & b \\ c & d \end{bmatrix}^{-1} = \frac{1}{ad - cb} \begin{bmatrix} d & -b \\ -c & a \end{bmatrix}$$

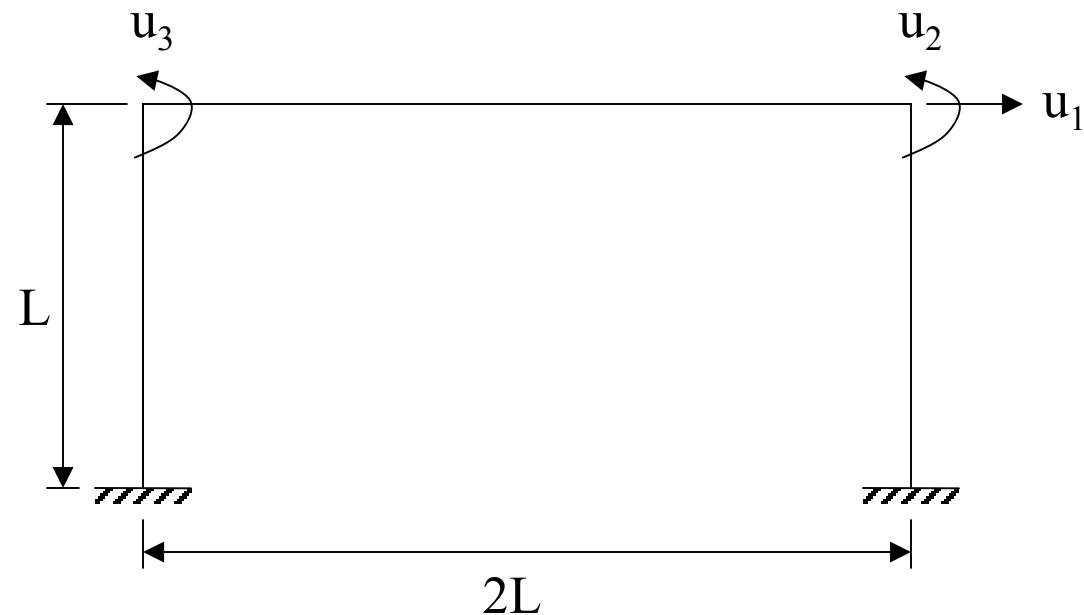
Substitute into 1<sup>st</sup> equation

$$\frac{EI}{L^3} \left[ 24 - \frac{36}{10} - \frac{36}{10} \right] u_1 = f_s = \frac{168EI}{10L^3} u_1$$

or

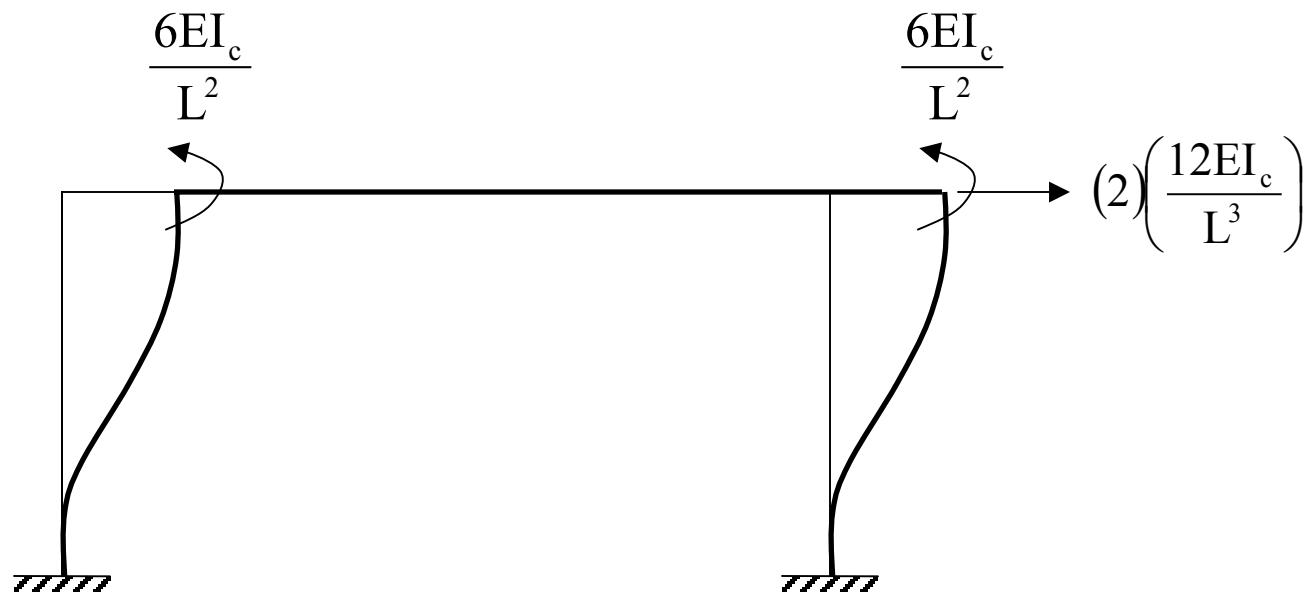
$$k = \frac{168EI}{10L^3} \quad (\text{check this result})$$

## Draft Example 2



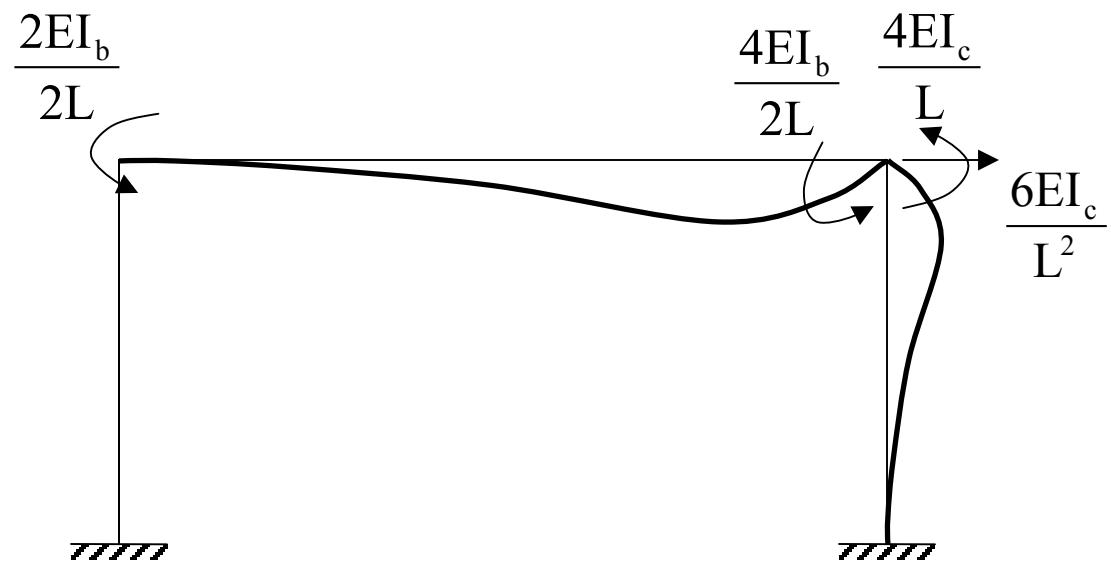
$$u_1 = 1$$

$$u_2 = u_3 = 0$$



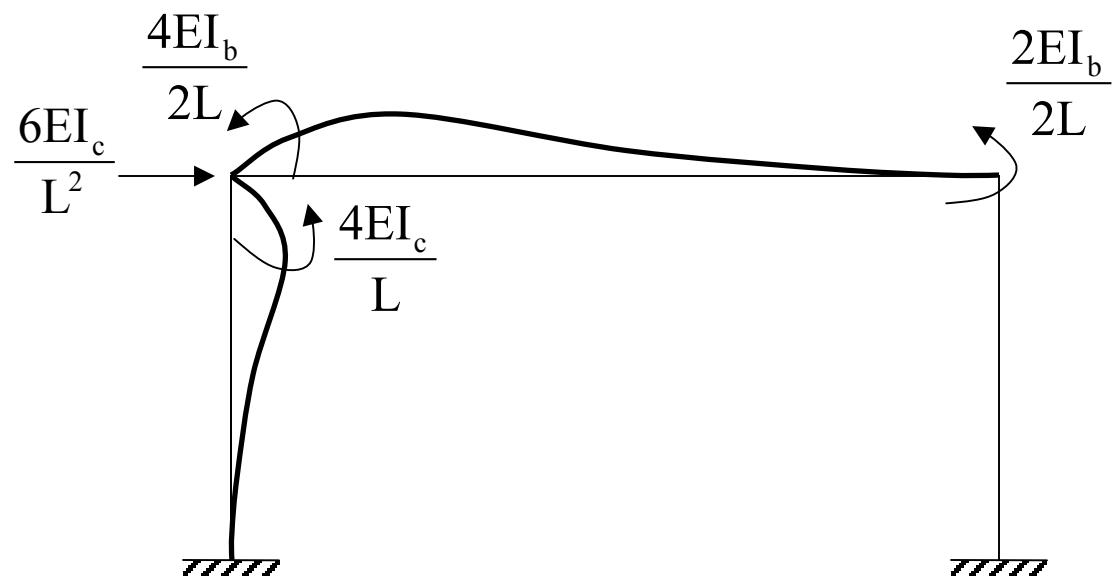
$$u_2 = 1$$

$$u_1 = u_3 = 0$$



$$u_3 = 1$$

$$u_1 = u_2 = 0$$



$$\underline{k} = \frac{E}{L^3} \begin{bmatrix} 24I_c & 6I_c L & 6I_c L \\ 6I_c L & 4\left(\frac{I_b}{2} + I_c\right)L^2 & I_b L^2 \\ 6I_c L & I_b L^2 & 4\left(\frac{I_b}{2} + I_c\right)L^2 \end{bmatrix}$$

For simplicity, let  $I_b = I_c$

$$\frac{EI}{L^3} \begin{bmatrix} 24 & 6L & 6L \\ 6L & 6L^2 & L^2 \\ 6L & L^2 & 6L^2 \end{bmatrix} \begin{bmatrix} u_1 \\ u_2 \\ u_3 \end{bmatrix} = \begin{bmatrix} f_s \\ 0 \\ 0 \end{bmatrix}$$

Static condensation:

$$\begin{bmatrix} u_2 \\ u_3 \end{bmatrix} = - \begin{bmatrix} 6L^2 & L^2 \\ L^2 & 6L^2 \end{bmatrix}^{-1} \begin{bmatrix} 6L \\ 6L \end{bmatrix} u_1$$

$$= \frac{-1}{36L^4 - L^4} \begin{bmatrix} 6L^2 & -L^2 \\ -L^2 & 6L^2 \end{bmatrix}^{-1} \begin{bmatrix} 6L \\ 6L \end{bmatrix} u_1$$

$$= \frac{-30}{35L} \begin{bmatrix} 1 \\ 1 \end{bmatrix} u_1 = \frac{-6}{7L} \begin{bmatrix} 1 \\ 1 \end{bmatrix} u_1$$

Substitute in 1<sup>st</sup> Equation

$$\frac{EI}{L^3} \left[ 24 - \frac{36}{7} - \frac{36}{7} \right] u_1 = f_s$$

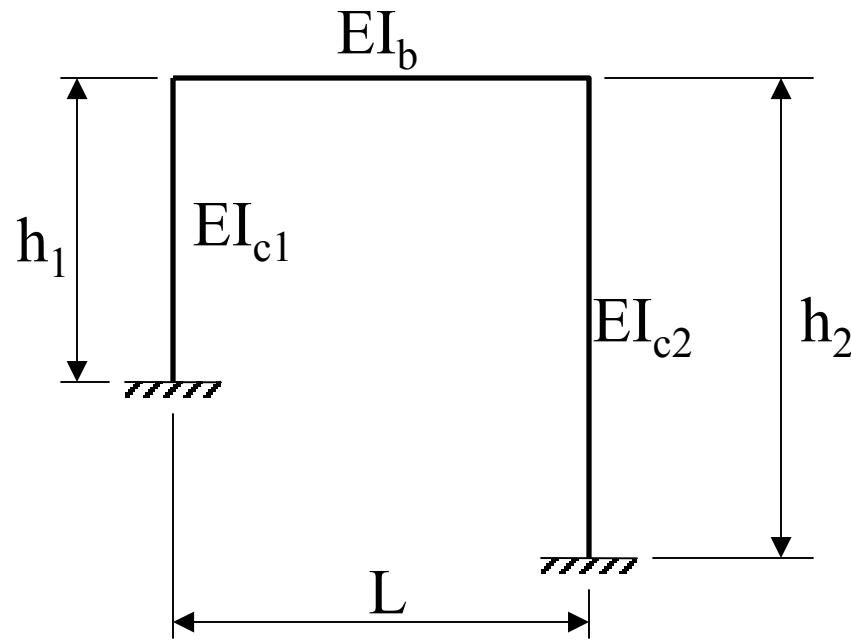
or,  $f_s = \frac{96}{7} \frac{EI}{L^3} u_1$

or,  $k = \frac{96EI}{7L^3}$

← Same as in Example 1.1,  
Dynamics of Structures by Chopra

## Sample Exercise

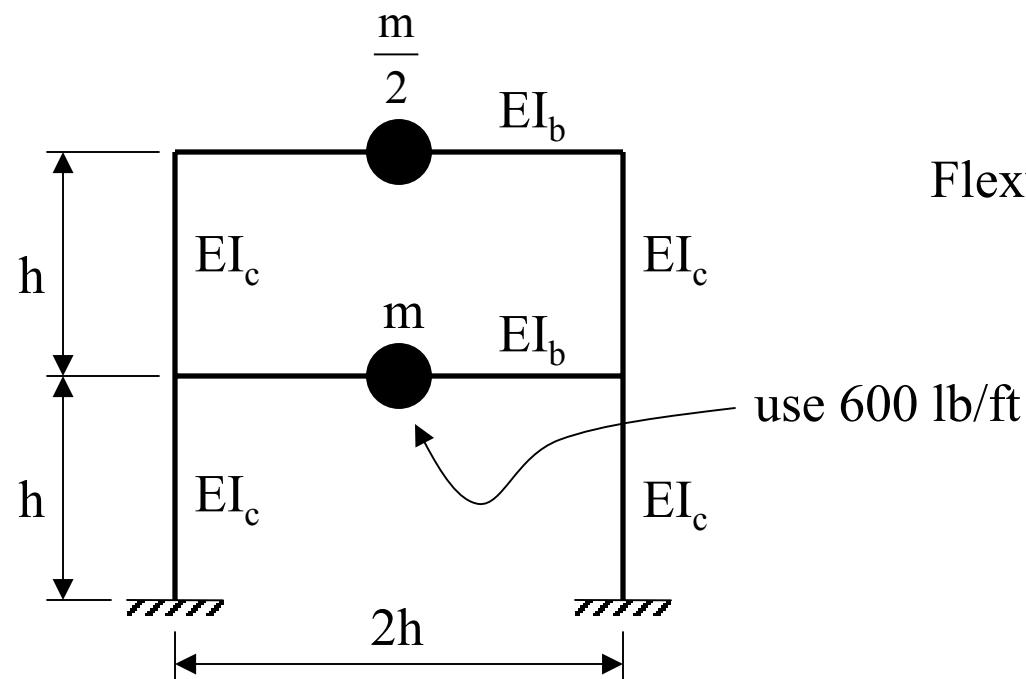
1) 1.1 Derive stiffness matrix  $\underline{k}$  for



1.2 For the special case of  $I_{c1} = I_{c2} = I_b$ ,  $h_1 = h_2 = h$  and  $L = 2h$ , find lateral stiffness  $k$  of the frame.

## Sample Exercise

2) Derive equation of motion for:



Flexural rigidity of beams and columns

$$E = 29,000 \text{ ksi},$$

Columns W8x24 sections

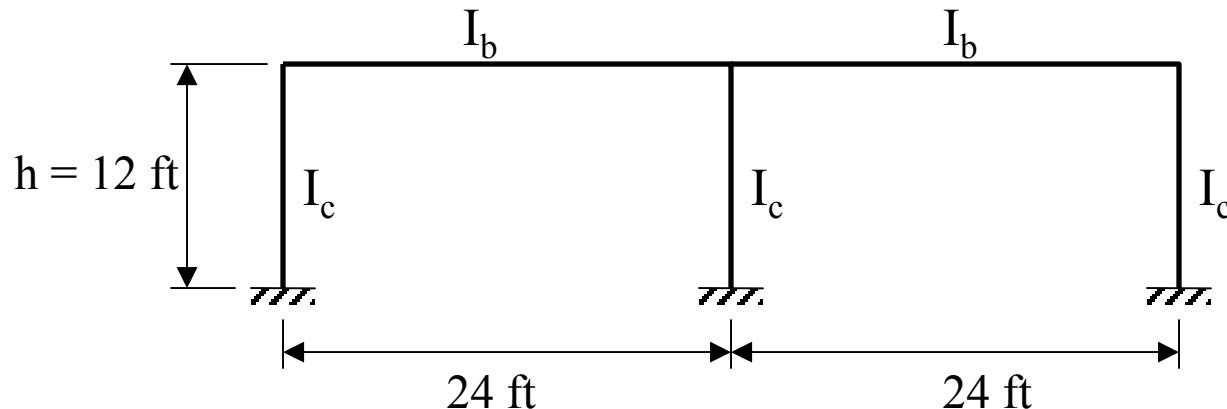
$$\text{with } I_c = 82.4 \text{ in}^4$$

$$h = 12 \text{ ft}$$

$$I_b = \frac{1}{2} I_c$$

### Sample Exercise (Optional)

3) Derive lateral k of system (need to use computer to invert 3x3 matrix)



$$E = 29,000 \text{ ksi},$$

$$I_c = 82.4 \text{ in}^4 \leftarrow \text{W8x24 sections}$$

$$I_b = \frac{1}{2} I_c$$